Inspection Report with SI&A Data

Milepoint: 0.020

Structure Description: 1020.01 Foot - 12 Span Steel continuous Stringer/Multi-beam or Girder

2 District: 05 **3 County:** Jefferson **16 Latitude:** 38°16′31.00″ 7 Longitude: 85°47′27.00″

7 Facility Carried 1-64

6A Feature Intersected: CONRAIL AND 26,27TH STS

NBI	Χ
Element	Χ
Fracture Critical	Χ
Underwater	
Special	

59 Superstructure: 6	62 Culvert: N										
60 Substructure: 7	Sufficiency Rating: 86.8										
DES	DESIGN										
Substandard:	No	00'									
Fracture Critical:	Yes										
43A Main Span Material:	(4) Steel Continuous										
43B Main Span Design:	(02) Stringer / Girder										
45 Number of Spans Main:	12	nge 00'									
44A Approach Span Material:	Not Applicable										
44B Approach Span Design:	Not Applicable										
46 Number of Approach Span	s: 0										
107 Deck Type:	(1) Concrete-Cast-in-Place										
108A Wearing Surface:	(6) Bituminous										
108B Membrane:	(0) None										
108C Deck Protection:	(0) None										
Overlay Y/N:	Yes										
Overlay Type:	HT Poly										
Overlay Thickness:	1.500 in										
Overlay Date:	2007										
	·										

	APPRA	AISAL
36A	Bridge Railings:	(1) Meets Standards
36B	Transitions	(1) Meets Standards
36C	Approach Guardrail:	(1) Meets Standards
36D	Approach Guardrail Ends:	(1) Meets Standards
71	Waterway Adequacy:	(N) Not Applicable
72	Approach Alignment:	(8) Equal Desirable Crit
113	Scour Critical:	(N) Not over Waterway
Reco	mmended Scour Critical:	(N) Not over Waterway

		LOAD RATINGS
63	Operating Type:	(1) Load Factor (LF)
64	Operating Rating:	60.0 tons
65	Inventory Type:	(1) Load Factor (LF)
66	Inventory Rating:	36.0 tons
Truck	Capacity Type I:	tons
Truck	Capacity Type II:	tons
Truck	Capacity Type III:	tons
Truck	Capacity Type IV:	tons

9 Location: .4M E OF K-ITRR OHIO RV B		Special
Structure Description: 1020 01 Foot - 12 Span Steel continuous S	tringer/Multi-beam or Girder	
NBI CONDITION RATINGS	OO" GEOM	ETRIC DATA
58 Deck: 7 61 Channel: N	48 Max Length Span:	133.000 ft
59 Superstructure: 6 62 Culvert: N	49 Structure Length:	1,020.084 ft
60 Substructure: 7 Sufficiency Rating: 86.8	32 Approach Roadway:	-3.281 ft
1 2 2	nge 33 Median:	(3) Closed w/Barrier
DESIGN	00" <mark>34 Skew:</mark>	0°
Substandard: No	35 Flare:	No Flare
Fracture Critical: Yes	50A Curb/Sidewalk Width L:	0.000 ft
43A Main Span Material: (4) Steel Continuous	50B Curb/Sidewalk Width R	: 0.000 ft
43B Main Span Design: (02) Stringer / Girder	47 Horiz. Clearance:	44.291 ft
45 Number of Spans Main: 12	nge 51 Width Curb to Curb:	-3.281 ft
44A Approach Span Material: Not Applicable	^{00″} 52 Width Out to Out:	94.250 ft
44B Approach Span Design: Not Applicable	48 Max Length Span:	133.000 ft
46 Number of Approach Spans: 0	ADMIN	NISTRATIVE
107 Deck Type: (1) Concrete-Cast-in-Place	27 Year Built:	1970
108A Wearing Surface: (6) Bituminous	106 Year Reconstructed:	0
108B Membrane: (0) None	42A Type of Service On:	(1) Highway
108C Deck Protection: (0) None	42B Type of Service Under:	(4) Hyw - RR
Overlay Y/N: Yes	37 Historical Significance:	(5) Not Eligible
Overlay Type: HT Poly	21 Maintenance Responsil	bility:(01) State Hwy Agency
Overlay Thickness: 1.500 in	22 Owner:	(01) State Hwy Agency
Overlay Date: 2007	101 Parallel Structure:	(N) No II Structure Exists
APPRAISAL	52 Width Out to Out:	94.250 ft
	CLE	ARANCES
36A Bridge Railings: (1) Meets Standards	10 Vert. Clearance:	23.419 ft
36B Transitions (1) Meets Standards	53 Min. Vert. Clearance Ov	ver: 99.999 ft
36C Approach Guardrail: (1) Meets Standards	54A Vert. Under Reference:	(H) Hwy beneath struct.

	POSTIN	IGS
10	Vert. Clearance:	99.999 ft
56	Min. Lat. Underclearance L:	0.000 ft
55B	Min. Lat. Underclearance R:	11.155 ft
55A	Lateral Under Reference:	(R) Railroad beneath struct.
54B	Min. Vert. Underclearance:	22.999 ft
54A	Vert. Under Reference:	(H) Hwy beneath struct.
53	Min. Vert. Clearance Over:	99.999 ft
10	Vert. Clearance:	23.419 ft

41 Posting Status:	(A) Open, No Restriction
Signs Posted Cardinal:	No
Signs Posted Non-Cardinal:	No
Field Postings Gross:	tons
Field Postings Type I:	tons
Field Postings Type II:	tons
Field Postings Type III:	tons
Field Postings Type IV:	tons

Inspection Report with SI&A Data

12: Re 0	Concrete Deck								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
SQ.FT	96,143	95,174	99%	961	1%	8	0%	0	0%

Deck was overlaid with Rosphalt in 2007 - top of the deck cannot be seen. The bottom of the deck has minor cracks, some with efflorescence. The bottom of the deck just west of Pier 9 has a spall with exposed rebar. The bottom of the east span has areas of delamination greater than 6 inch diameter near the centerline joint.

510: We	earing Surfaces								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
SQ.FT	90,447	89,966	99%	481	1%	0	0%	0	0%

Deck was overlaid with Rosphalt in 2007. Rosphalt has exposed aggregate in the driving lanes, but appears to be fully effective at protecting the deck. Rosphalt has some small areas of scattered nicks/gouges that appear to be from impact (estimated at 0.5% of the surface, but that may be a bit large). The armored edge and the top of the backwall at the east end of the westbound side has moderate to fairly severe impact damage from snowplows. Dirt/debris littering the shoulders - northwest corner of the bridge has lots of dirt build-up (worst area).

107: Steel Opn Girder/Beam									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	10,469	9,434	90%	1,020	10%	15	0%	0	0%

Beams have welds that vary in quality from good to poor and intersecting welds are typically scattered throughout the beams. Longitudinal beams have some section loss under joints and light freckled rust for approximately 10% of their length. Cross girders at Piers 2, 3, and 4 have about 5 feet each of rusted areas with minor section loss under the centerline joint.

515: Steel Protective Coating									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	3,190.95	0	0%	2,871.83	90%	319.13	10%	0	0%

Paint is mostly substantially effective, but has areas of only limited effectiveness. The paint on this structure (as well as the remainder of the paint on the Riverside Expressway bridges) is an overcoat system that is very near the end of its life cycle - planning for new paint needs to begin ASAP.

Inspection Report with SI&A Data

161: Stl Pin Pin/Han both									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
EACH	10	0	0%	10	100%	0	0%	0	0%

The wind locks on bottom and top flanges of suspended beams have section loss/bending/deterioration. Some assemblies typically have contact between girder web and hanger plate and some have minor section loss behind hanger plates. Some hangers, pins, and nuts have minor nicks/gouges that appear to be from construction. The presence of fretting corrosion at some assemblies indicate that the connections are allowing movement.

515: Ste	515: Steel Protective Coating											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4			
EACH	0.93	0	0%	0	0%	0.93	100%	0	0%			

Calcium sulfonate paint utilized on the pin and hangers during the 2007 rehab project provides only limited effectiveness at best.

205: Re Conc Column										
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4	
EACH	40	29	73%	9	23%	2	5%	0	0%	

Pier 1 Column 3 has a minor spall near the ground. Pier 2 Column 1 has minor deterioration and Column 2 has a minor spall with exposed resteel near the top. Pier 3 Columns 2 and 3 have minor spalls. Pier 4 Column 2 has minor deterioration near the top. Pier 5 Column 3 has cracking with some delamination.

215: Re	Conc Abutment								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	189	141	75%	48	25%	0	0%	0	0%

Abutments have minor cracks, deterioration, and spalls. Abutment backwalls have vertical and random cracks.

Inspection Report with SI&A Data

234: Re	Conc Pier Cap								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	748	680	91%	45	6%	23	3%	0	0%

Some pier caps have minor to moderate cracks and deterioration - Piers 5 and 8 mostly. Minor to moderate cracking and/or spalling of some pedestals. Pier 5 cap has a minor spall with exposed rebar near the south end on the west face. Pier 8 pedestal for Beam A is spalled with exposed rebar on the west face.

300: Strip Seal Exp Joint											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4		
FT	355	285	80%	70	20%	0	0%	0	0%		

All joints replaced in 2007. Strip seals have minor dirt/debris build-up in them and concrete adjacent to the armored edges has minor cracks/deterioration. Pier 5 and End Bent 2 joints have minor damage from snowplows.

302: Compressn Joint Seal											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4		
FT	89	49	55%	15	17%	25	28%	0	0%		

All joints replaced in 2007. Compression seal at the west bridge end (End Bent 1) has minor dirt/debris build-up and concrete adjacent to armored edges has minor cracks/deterioration. The compression seal in the eastbound lanes has dropped down in places.

311: Mo	oveable Bearing								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
EACH	85	0	0%	85	100%	0	0%	0	0%

Moveable bearings have minor surface rust.

Inspection Report with SI&A Data

515: Ste	515: Steel Protective Coating											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4			
EACH	7.9	0	0%	4.18	53%	3.72	47%	0	0%			

Paint of bearings at the end bents, Pier 5, and Pier 8 has limited effectiveness - at other bearings it is still substantially effective.

313: Fixed Bearing											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4		
EACH	43	0	0%	41	95%	2	5%	0	0%		

Bearings appear to have been reset, cleaned, and painted as part of repairs performed in 2007. Fixed bearings at Piers 3 and 4 have rust with minor section loss under the centerline joint.

515: Steel Protective Coating											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4		
EACH	3.99	0	0%	3.99	100%	0	0%	0	0%		

Paint at the fixed bearings is substantially effective.

331: Re	331: Re Conc Bridge Railing											
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4			
FT	4,080	3,629	89%	401	10%	50	1%	0	0%			

Exterior bridge railing was retrofitted to meet current standards in 2007. Median bridge railing repaired and masonry coated in 2007. Concrete barrier wall has minor vertical cracks, some minor spalls with exposed rebar, and a few scrapes from vehicle impact.

Inspection Report with SI&A Data

802: Drainage Sys										
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4	
EACH	1	0	0%	0	0%	0	0%	1	100%	

Drainage system is clogged up and/or rusted out.

807: Stl Cross Girder									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(LF)	269	254	94%	0	0%	15	6%	0	0%

Steel cross girders have expansive rust with some minor section loss under the centerline longitudinal joint. (This 269 feet of girder is also included in Element 107.)

515: Steel Protective Coating									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(LF)	81.99	0	0%	77.42	94%	4.57	6%	0	0%

Paint on the cross girders is substantially effective, except under the centerline joint where it now has only limited effectiveness.

851: Transitions									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	0	0%	0	0%	1	100%

The armored edge and the top of the backwall at the east end of the westbound side has moderate to fairly severe impact damage from snowplows - the approach at this location needs a large asphalt wedge.

Inspection Report with SI&A Data

852: Drains									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	0	0%	1	100%	0	0%

Drains connected to a drainage system are stopped up.

857: Embankment Erosion									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	0	0%	0	0%	1	100%

The east abutment (End Bent 2) has fairly severe erosion at the south side of it and in front of it near the south side - the slope protection is undermined approximately 4 feet. Minor erosion in front of the west abutment (End Bent 1). Moderate to severe roadway embankment erosion on the right side of the eastbound lane embankment approximately 40 feet east of the east bridge end.

859: Vegetation									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	0	0%	1	100%	0	0%

Trees and brush around the structure need to be cut, sprayed, and removed.

Inspector - TGKING (163)

Inspection Report with SI&A Data

STRUCTURE NOTES

- -Since this bridge has fracture critical pier-girders and Pier-Girder 2 has pin and hanger assemblies, the decision was made that bridge component numbering would be designated as per original plans (contrary to our Kentucky Bridge Inspection Procedures Manual). Therefore bridge component numbering in inspections prior to 2012 may vary from newer inspections.
- -In 2007, deck received 1.5" Rosphalt overlay (high temp.) and joint replacement.
- -To access, see contact information for Norfolk Southern RR (needs 3 days notice; no work on Friday):
- -Carlene Ward, National Contact at (865) 521-1481
- -lan Krispin, Regional Contact at (217) 619-4551
- -Jim Kovats, Local Contact at (502) 442-6991
- -In-depth inspection was performed on this structure in August of 1989.
- Element " 107 Steel Painted Open Girder/Beam" includes 269 feet of steel cross girder. These steel cross girders are fracture critical and are located at Piers 2, 3, and 4. Central Office instructed us to code them like this.
- -This structure has longitudinal stiffeners. Shop drawings are in the file.

INSPECTION NOTES

Terry King, Natalie House-Lewis, and James Whitehouse performed the fracture critical portion of the bridge inspection on July 21 and 22, 2015. Access for inspection of the fracture critical cross girders and the pin and hanger assemblies was obtained with the District's 65' self propelled man lift. The fracture critical portion of the inspection could possibly be completed in one day, but two days were required this cycle due to having to wait for the rail line to be cleared the first morning and attending a meeting after lunch on the second day. Norfolk Southern personnel provided RR "flagging" while inspection was performed on the portions of the cross girders located above the RR tracks.

Terry King and Taylor Hancock performed the routine portion of the inspection on 7-23-2015.

The fracture critical inspection included hands-on visual inspection of the three (3) cross girders as well as the pin & planer assemblies on the west face of the Pier 2 cross girder. Magnetic-particle testing equipment was available for use, but was not required.

The fracture critical cross girders were found to be in generally satisfactory condition, with only minor deterioration/defects. The cross girders are rusting in places and have experienced some minor section loss below the longitudinal deck joint.

The pin and hanger assemblies have minor pack rust and deterioration in places. Some pin and hanger locations also exhibit minor section loss, although most areas of section loss have been painted over. The longitudinal girders have minor to moderate section loss to the web/lower flange near the pin and hanger assemblies - with Beam A having the worst section loss. Steel cover plates that prevented good visual inspection of the pin and hangers on the exterior of the fascia girders were removed at the end of the 2013 inspection - after inspecting this cycle, touch-up painting was completed at these locations.

	WORK
Action:	